TECHNICAL PRODUCT INFORMATION





- DIBt Approval Z-21.8-2035
- Type tested acc. to approval



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CAD drawings for HBT-Elements can be found in the HALFEN CAD library under reinforcement technology. All drawing can be downloaded free at www.halfen.com. Following file formats are available:

• CAD - DWG and DXF

A free DVD is also available if preferred. Our contact details, addresses, telephone and fax numbers can be found on the back cover of this catalogue Following approval and type test apply in this document:

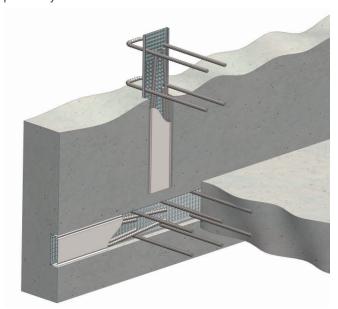
- German National Technical Approval (DIBt Deutsches Institut f
  ür Bautechnik) No. Z-21.8-2035 for HALFEN HBT Rebend Connection
- Type test report no. 4117-6131/14 for HALFEN HBT Rebend Connection according to approval no. Z-21.8-2035



Introduction and System Description

#### **HBT** - the connection solution

The HALFEN HBT is the first rebend connection with general building approval. The simplified calculation method according to the approval requires verification of only two basic load cases, this results in higher shear load capacity than previously.



#### Safety

- improved planning-reliability through general building authority approval based on real-world load capacity tests
- type tested load capacity tables
- more safety in planning and execution due to factory production and third party control

# Simple

- simplified calculation concept with only two basic load cases
   shear load longitudinal to joint
  - shear load transverse to joint
- both shear and longitudinal load transfer using standard type profiles
- if required a verification of combined shear and longitudinal load in the concrete joint is possible
- standard type is suitable for constructive connections as well as for static critical connections – no risk of mix ups on-site or in the precast plant
- reduced case height ideal for thin elements or precast concrete elements with minimal concrete cover

HALFEN HBT Rebend connections allow a simple and effective connection of reinforced concrete elements, which are cast in different phases. All types of slabs, from floor slabs, walls and stairs can be subsequently cast with sufficient bond.

The rebar are bent and secured in a case with a back and a removable cover. The HALFEN profile cases are available in different widths. The case is cast into the concrete element; after striking the formwork, the case cover is removed and the rebar is straightened.

The customer can select either a single-row or double-row profile. The single-row profile has a regular spacing of holes in a single row penetrated by reinforcement bars designed for the required application; the double-row profile is similar but has two rows.

- rebar (8, 10, 12 mm)
   B500B steel (stainless steel
   B500 NR on request)
- the back is galvanised sheet metal with a specially profiled surface
- dimensionally stable, galvanised, sheet metal cover with a pre-punched hole to facilitate removal



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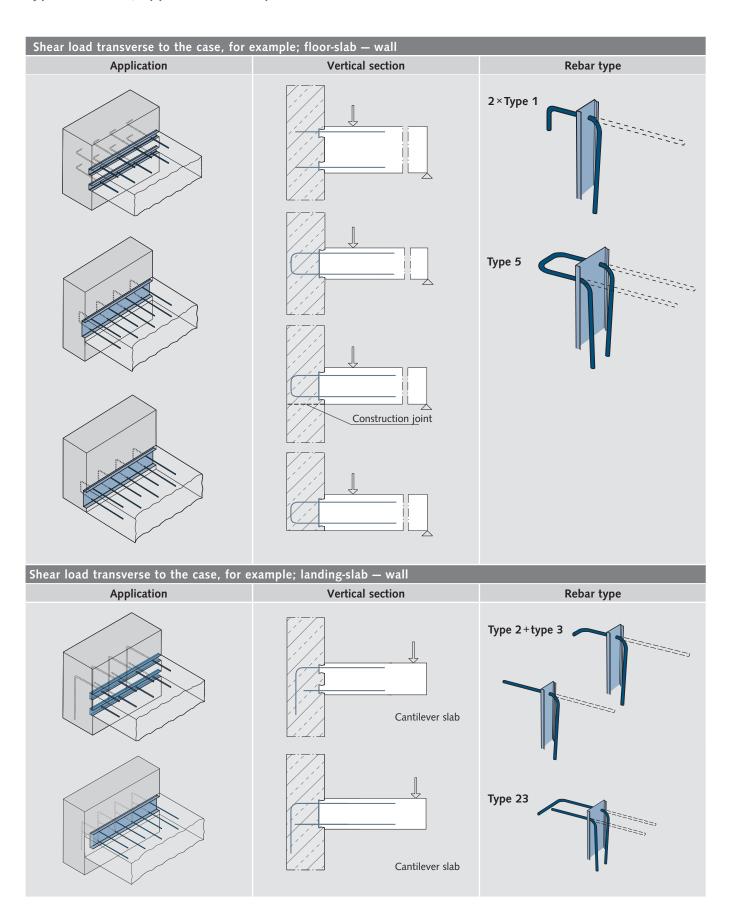
#### Versatile

Optimal connections for a multitude of applications with 57 possible combinations of rebar and profile widths. A standardized range of product with element lengths of 0.8 m and 1.25 m are available for the most common applications.

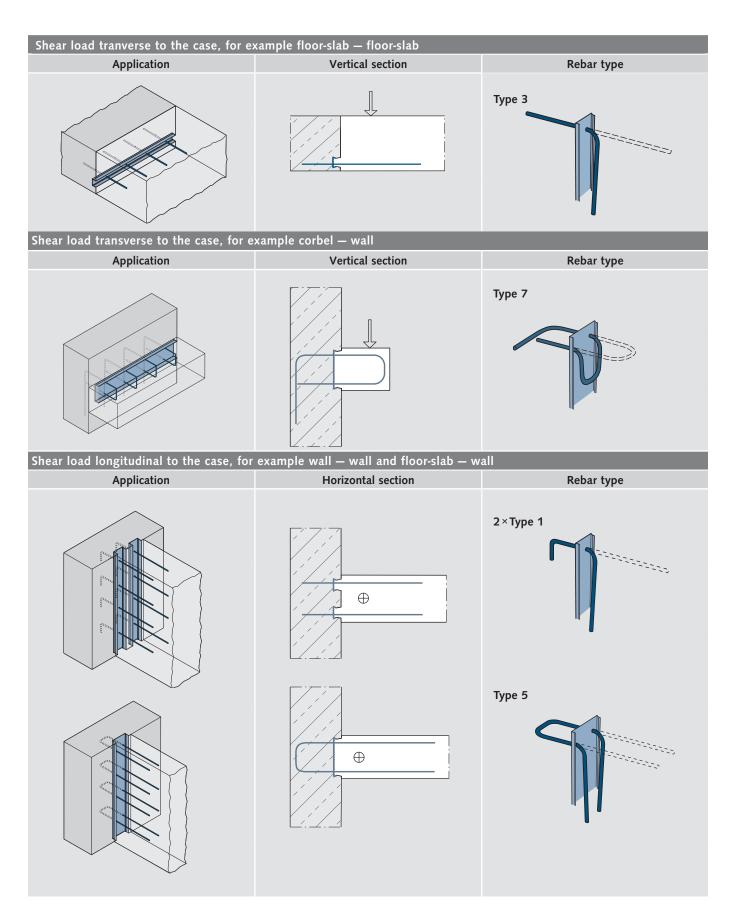
Lots of reasons, one conclusion: safety, quality and protection - for you and your company.



Type Overview/Application Examples

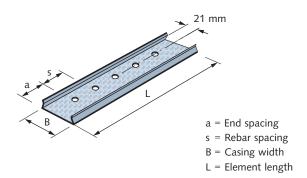


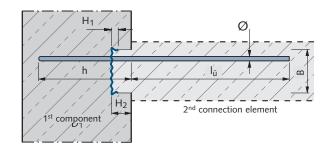
# Type Overview/Application Examples



# Single-Row Profiles

# Profile dimensions



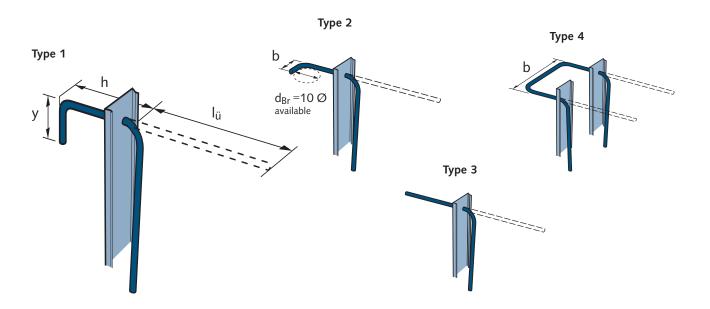


Case dimensions [mm]									
profile	rebar Ø [mm]	width B	height H <sub>1</sub>	height H <sub>2</sub>					
HBT 55	8	58		24					
HB1 99	10	ەد		30					
HBT 85	10	86	12	30					
1101 65	12	80	12	36					
HBT 120	10	122		30					
HB1 120	12	122		36					

Rebar layout			
element length L	rebar spacing s [cm]	number of rebar	end spacing a [cm]
	10	12	7.5
standard element	15	8	10.0
L = 1250 mm	20	6	12.5
	25	5	12.5
	10	8	5
short element	15	6	2.5
L = 800 mm	20	4	10
	25	4	2.5

Other element lengths on request

# Rebar types



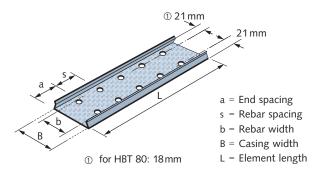
See table on page 7 for rebar dimensions

# Single-Row Profiles

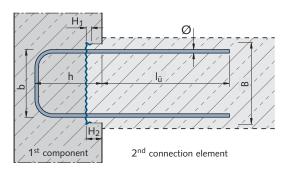
ebar dimensions									
profile	rebar Ø	rebar spacing		standard type 1			types 1, 2, 3, 4		only for type
				L = 1250 mm		l.	L = 1250 mm	L ≥ 800 mm	
	[mm]	[cm]	h	Ι <sub>ü</sub>	у	h <sub>min</sub>	I <sub>ü,max</sub>	I <sub>ü,max</sub>	b [mm]
HBT 55		10		210	75		210	210	
	8	15	170			120	510	430	
		20	170	320			600	455	
		25					600	480	200500
		10		200			200	200	200300
	10	15	170	390	95	120	390	390	
	10	20	170			120	510	450	
		25					510	475	
HBT 85	10	10					430	400	
		15	170	390	95	120	510	425	
		20	170	390	95	120	600	450	
		25					000	475	250500
		10		430	110	120	430	395	
	12	15	170	460			510	420	
	12	20	170				600	445	
		25					000	470	
HBT 120		10						400	
	10	15	170	390	95	120	600	425	
	10	20	170	390	93	120	000	450	
		25						475	on request
		10						395	
	40	15	170	460	110	) 120	600	420	
	12	20	170	400	110			445	
		25						470	

#### **Double-Row Profiles**

# Profile dimensions



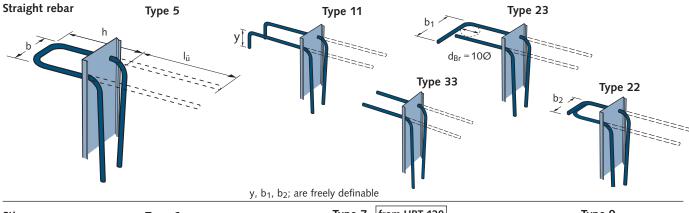
Case dimer	nsions [mm]				
profile	rebar Ø [mm]	width B	height H <sub>1</sub>	height H <sub>2</sub>	b
HBT 80	8	0.0		24	58
пвт во	10	86		30	60
	8			24	88
HBT 120	10	122		30	90
	12			36	92
	8			24	116
HBT 150	10	150	12	30	118
	12		12	36	120
	8			24	152
HBT 190	10	186		30	154
	12			36	156
	8			24	188
HBT 220	10	222		30	190
	12			36	192

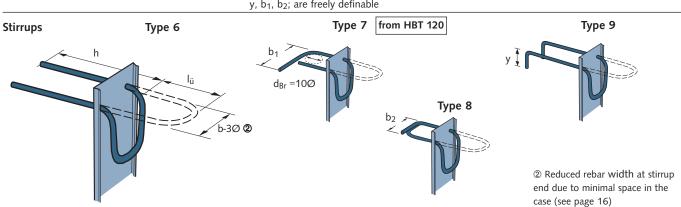


Rebar layout			
element length L	rebar spacing s [cm]	number of rebar	end spacing a [cm]
	10	12	7.5
standard element	15	8	10.0
L = 1250 mm	20	6	12.5
	25	5	12.5
	10	8	5
short element	15	6	2.5
L = 800 mm	20	4	10
	25	4	2.5

Other element lengths on request

# Rebar types





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# **Double-Row Profiles**

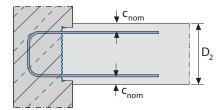
profile	robar (X	rebar	standard	type 5	+	es 5, 11, 22, 23	@ 33		types 6,	7 9 0									
profile	rebarØ	spacing		type 5	тур			t			1 > 600								
			L=1250 mm			L=1250 mm		type 6	type 7 ③, 8	type 9	L≥ 600 mm								
	[mm]	[cm]	h	Ιü	h <sub>min</sub>	I <sub>ü,max</sub>	I <sub>ü,max</sub>	h <sub>min</sub>	h <sub>min</sub>	h <sub>min</sub>	60≤l <sub>ü</sub> ≤l <sub>ü,ma</sub>								
HBT 80 3		10		170		170	170				70								
	8	15	170		120	360	360	120	135	125 110	125								
	0	20	170	320	120	450	450	120	135	140	175								
		25				500	480				225								
		10		160		160	160				60								
	10	15	170	320	120	320	320	120	120	120	120	120	120	120	155	155	155	140	120
	10	20	170	390	120	400	400					155	199	140	170				
		25		390		450	450				220								
HBT 120		10		290		290	290				115								
	8	15	170		120	510	430	120	135	140	175								
<b>.</b>	ō	20	170	320	120	600	455	120	133	140	235								
		25				600	480				290								
		10		240		240	240				80								
_	40	15	470		420	510	425	420	455	4.40	130								
	10	20	170	390	120	600	450	120	155	140	180								
		25				600	475				230								
		10		215		215	215				70								
		15		390		390	390				120								
	12	20	170	440	120	440	440	120	170	140	170								
		25		460		490	470				220								
HBT 150		10				360	360				100								
1101 130		15				510	430				150								
	8	20	170	320	120		455	120	135	140	210								
		25				600	480				260								
		10		360		360	360				85								
		15				510	425				135								
	10	20	170	390	120	310	450	120	155	140	185								
		25		370		600	475				235								
		10		310		310	310				90								
		15		310		480	420				150								
	12	20	170	460	120	530	445	120	170	140	200								
		25		400		580	470				250								
LIDT 400		10				500	405				100								
HBT 190		15				510	430				150								
	8	20	170	320	120	310	455	120	135	140	210								
		25				600	480				260								
		10				500	400												
		15				500	400				110 170								
	10		170	390	120	510		120	155	140									
		20				600	450				220								
		25		420		420	475				280								
		10		430		430	395				90								
	12	15	170	460	120	510	420	120	170	140	140								
		20		460		600	445				190								
		25					470				240								
HBT 220		10					405				100								
	8	15	170	320	120	600	430	120	135	140	150								
		20					455				210								
		25					480				260								
		10					400				110								
	10	15	170 390 170 600 170 155 17	140	170														
~		20					450				220								
		25					475				280								
		10					395				90								
	12	15	170	460	120	600	420	120	170	140	140								
	20	20		.00	.20	555	445	3			190								
		25					470				240								

 $<sup>\</sup>textcircled{3}$  rebar type 7 for HBT 80 not available 4 Due to the required bending roll diameter (d<sub>Br</sub> = 10x bar diam.); h<sub>min</sub> for type 23 is equal to h<sub>min</sub> of type 7

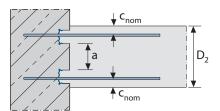
Product Selection/Calculation Basis according to Approval no. Z-21.8-2035

Product selection all	Product selection allowing for required concrete cover of the rebend reinforcement											
		Thickness of component D <sub>2</sub> [mm]										
profile	100	120	140	160	180	200	220	240	260	280	300	
		concrete cover c <sub>nom</sub> [mm]										
2×HBT 55 ①	-	-	≤ 25	≤ 35	≤ 45	≤ 55	≤ 65	≤ 75	≤ 85	≤ 95	≤ 105	
2×HBT 85 ①	-	-	-	-	≤ 20	≤ 30	≤ 40	≤ 50	≤ 60	≤ 70	≤ 80	
HBT 80	20	30	40	50	60	70	80	90	100	110	120	
HBT 120	-	-	25	35	45	55	65	75	85	95	105	
HBT 150	-	-	-	20	30	40	50	60	70	80	90	
HBT 190	-	-	-	-	-	23	33	43	53	63	73	
HBT 220	-	-	-	-	-	-	-	25	35	45	55	

① depends on spacing a between the cases



Product selection for 1-part element



Product selection for multi-part elements

#### Basis for calculation according to approval number Z-21.8-2035

# General information

The concept for the approval is based on the calculation and the structural application as applied in the following standards and guidelines: DIN EN 1992-1-1 with DIN EN 1992-1-1/NA (National Annex) and the DBV-guidelines "Rebending of reinforcing steel and requirements of protective boxes according to Eurocode 2". Generally two different cases of shear load are examined: shear load transverse and longitudinal to the concrete joint.

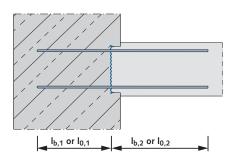
According to the approval, due to the product characteristics the shear loads transverse to the joint in the HBT Rebend connections can be classed as "indented" and shear loads longitudinal to the joint can be classified as "rough".

Material: Rebend reinforcement B500B, stainless steel
B500 NR with bar diameters of 8, 10 and 12 mm
Normal concrete ≥ C20/25

The maximum load bearing capacity of the rebend connection must be limited to 80% of the ultimate limit state; the following applies for tensile strength:

$$f_{yd,red} = 0.8 \cdot \frac{f_{yk}}{\gamma_s}$$

Existing anchorage lengths and overlap lengths must be taken into consideration for the calculation. These can be calculated from the back surface of the case. Verification of the anchorage lengths and overlap lengths is according to Eurocode 2, taking the bonding characteristics into account.



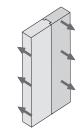
Anchorage and overlap length requirements for the reinforcement

Calculating the HALFEN HBT Rebend connections as in the DBV-guidelines "Rebending of reinforcing steel and requirements of protective boxes according to Eurocode 2" with roughness classification "smooth", is conservative while still being acceptable.

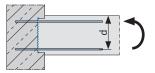
Calculation Basis according to Approval no. Z-21.8-2035

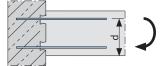
#### Shear load, transverse to the concrete joint

Calculation is according to DIN EN 1992-1-1, section 6.2 and DIN EN 1992-1-1/NA, as for monolithic produced building components; whereby the following additional provisions must be observed.



Effective static height:

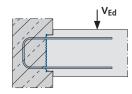




Tension zones: upper and lower component edges

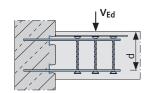
#### Shear resistance with no shear reinforcement

The decisive resistance  $V_{Rd,c}$  for the calculation is according to DIN EN 1992-1-1, section 6.2.2; whereby a reduction in the reinforcement ratio  $\rho_l$  is not required (caused by the reduced yield strength of the rebend reinforcement).



#### Shear resistance with shear reinforcement

The decisive resistance for verification results from 30% of the shear load resistance V<sub>Rd,max</sub> according to DIN EN 1992-1-1, section 6.2.3:



 $V_{Ed} \le V_{Rd} \le 0.3 \cdot V_{Rd,max}$ 

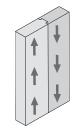
An additional load in the longitudinal reinforcement caused by shear forces must be verified assuming a compression strut angle of 45° (cot  $\theta$  = 1.0). The required shear reinforcement is calculated using cot  $\theta$  = 1.0

To calculate the shear load reinforcement for HALFEN HDB-S Shear rails see HDB technical product information and the approvals Z-15.1-249 and Z-15.1-270.

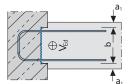
# Shear load, longitudinal to the concrete joint

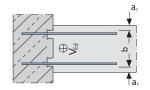
Static verification is according to DIN EN 1992-1-1, section 6.2.5 and DIN EN 1992-1-1/NA (National Annex), whereby the following additional provisions are to be observed.

Factors to used to calculate the shear load resistance:



c = 0.4  $\mu = 0.7$ v = 0.5





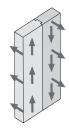
If the spaces between the HBT Cases are additionally subjected to shear load, then these must be designed as rough or suitably indented in accordance with DIN EN 1992-1-1. The edge areas can also be assumed as load bearing if  $a_1 \ge 50 \, \text{mm}$ .

The concrete cover  $c_{nom}$  is according to DIN to EN 1992-1-1. In addition for the rebend reinforcement the following must be observed.

 $c_{nom} \ge max. \{3 \varnothing, 30 mm, max. aggregate diameter d_g\}$ 

# Combined shear load, transverse and longitudinal to the concrete joint

If the connection is subjected to combined shear load (longitudinal and transverse to the concrete joint), each load direction can be verified separately.



#### TECHNICAL CONSULTATION

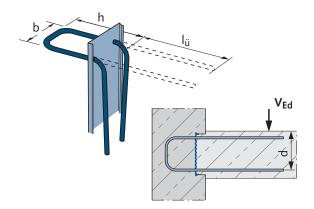
#### **HALFEN Technical services**

See back of catalogue for technical advice for your individual projects and contact information for all products.

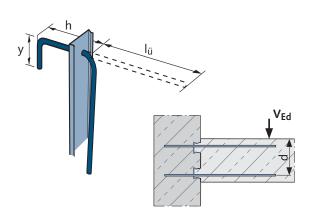
Selected Load Capacity Values according to Type Test and Approval no. Z-21.8-2035

# Shear load capacity transverse to the case — no shear reinforcement

Single connection example;  $1 \times Type 5$ 



Multipart connection example; 2 × Type 1



Shear load resi	istance V <sub>Rd</sub> [kN	I/m] (≤ 0.3·V	Rd,max)				Alwa	ys refer to th	e information ir	n the type test
rebar Ø/	d = 100 mm	d = 120 mm	d = 140 mm	d = 160 mm	d = 180 mm	d = 200 mm	d = 220 mm	d = 240 mm	d = 260 mm	d = 280 mm
spacing s [mm/cm]	HBT 80	HBT 120	HBT 150	HBT 150	HBT 190	HBT 190	HBT 220	HBT 220	2×55(Ø8) 2×85(Ø10,12)	2×55(Ø8) 2×85(Ø10,12)
				concrete	strength cla	ss C20/25				
8/20	44.3	53.1	62.0	70.8	78.7	78.7	78.7	78.7	78.7	78.7
8/15	44.3	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	104.9
8/10	-	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
10/20	44.3	53.1	62.0	70.8	79.7	88.5	94.0	98.4	98.4	98.4
10/15	44.3	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
10/10	-	56.6	62.7	70.8	79.7	88.5	94.0	99.4	104.7	109.8
12/20	-	53.1	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
12/15	-	55.8	62.0	70.8	79.7	88.5	94.0	99.4	104.7	109.8
12/10	-	63.9	70.8	77.4	83.7	89.8	94.0	99.4	104.7	109.8
				concrete	strength cla	ss C25/30				
8/20	49.5	59.4	69.3	78.7	78.7	78.7	78.7	78.7	78.7	78.7
8/15	49.5	59.4	69.3	79.2	89.1	99.0	104.9	104.9	104.9	104.9
8/10	-	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
10/20	49.5	59.4	69.3	79.2	89.1	99.0	105.1	111.1	114.2	114.2
10/15	49.5	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
10/10	-	60.9	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
12/20	-	59.4	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
12/15	-	60.1	69.3	79.2	89.1	99.0	105.1	111.1	117.0	122.8
12/10	-	68.8	76.3	83.4	90.2	99.0	105.1	111.1	117.0	122.8
				concrete	strength cla	ıss C30/37				
8/20	54.2	65.1	75.9	78.7	78.7	78.7	78.7	78.7	78.7	78.7
8/15	54.2	65.1	75.9	86.8	97.6	104.9	104.9	104.9	104.9	104.9
8/10	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
10/20	54.2	65.1	75.9	86.8	97.6	108.4	115.1	121.7	122.9	122.9
10/15	54.2	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
10/10	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
12/20	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
12/15	-	65.1	75.9	86.8	97.6	108.4	115.1	121.7	128.2	134.5
12/10	-	73.1	81.0	88.6	97.6	108.4	115.1	121.7	128.2	134.5

Note: Standard dimension according to page 7 and 9. Load capacities for further rebar dimensions and for rebar spacings of 25 cm, see type test.

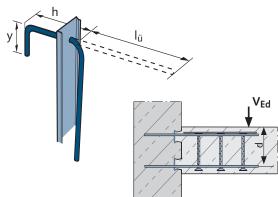
12

Selected Load Capacity Values according to Type Test and Approval no. Z-21.8-2035

# Shear load capacity transverse to the case — with shear reinforcement

Single connection example; 1 × Type 5





Multipart connection example; 2 × Type 1

Please refer to the HDB Product information and approvals no. Z-15.1-249 and Z-15.1-270 when determining the shear reinforcement using HALFEN HDB-S Shear reinforcement.

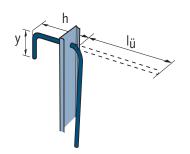
Shear load re	esistance V <sub>Rd</sub> [k	:N/m] (≤ 0.3·	V <sub>Rd,max</sub> )				Alwa	ys refer to th	e information ir	the type test
rebarØ/	d = 100 mm	d = 120 mm	d = 140 mm	d = 160 mm	d = 180 mm	d = 200 mm	d = 220 mm	d = 240 mm	d = 260 mm	d = 280 mm
spacing s [mm/cm]	HBT 80	HBT 120	HBT 150	HBT 150	HBT 190	HBT 190	HBT 220	HBT 220	2×55(Ø8) 2×85(Ø10,12)	2×55 (Ø 8) 2×85 (Ø 10,12
				concrete	strength cla	iss C20/25				
8/20	76.5	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4
8/15	76.5	102.0	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6
8/10	-	102.0	127.5	153.0	174.8	174.8	174.8	174.8	136.5	136.5
10/20	76.5	102.0	109.4	109.4	109.4	109.4	109.4	109.4	109.4	109.4
10/15	76.5	102.0	127.5	145.8	145.8	145.8	145.8	145.8	145.8	145.8
10/10	-	102.0	127.5	153.0	178.5	204.0	218.7	218.7	218.7	218.7
12/20	-	102.0	125.6	125.6	125.6	125.6	125.6	125.6	125.6	125.6
12/15	-	102.0	127.5	153.0	167.5	167.5	167.5	167.5	167.5	167.5
12/10	-	102.0	127.5	153.0	178.5	204.0	229.5	251.2	251.2	251.2
				concrete	strength cla	iss C25/30				
8/20	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4
8/15	95.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6
8/10	-	127.5	159.4	174.8	174.8	174.8	174.8	174.8	158.4	158.4
10/20	95.6	126.9	126.9	126.9	126.9	126.9	126.9	126.9	126.9	126.9
10/15	95.6	127.5	159.4	169.2	169.2	169.2	169.2	169.2	169.2	169.2
10/10	-	127.5	159.4	191.3	223.1	253.8	253.8	253.8	253.8	253.8
12/20	-	127.5	145.8	145.8	145.8	145.8	145.8	145.8	145.8	145.8
12/15	-	127.5	159.4	191.3	194.4	194.4	194.4	194.4	194.4	194.4
12/10	-	127.5	159.4	191.3	223.1	255.0	286.9	291.5	291.5	291.5
				concrete	strength cla	iss C30/37				
8/20	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4	87.4
8/15	114.8	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6	116.6
8/10	-	153.0	174.8	174.8	174.8	174.8	174.8	174.8	174.8	174.8
10/20	114.8	136.6	136.6	136.6	136.6	136.6	136.6	136.6	136.6	136.6
10/15	114.8	153.0	182.1	182.1	182.1	182.1	182.1	182.1	182.1	182.1
10/10	-	153.0	191.3	229.5	267.8	273.2	273.2	273.2	273.2	273.2
12/20	-	153.0	164.6	164.6	164.6	164.6	164.6	164.6	164.6	164.6
12/15	-	153.0	191.3	219.5	219.5	219.5	219.5	219.5	219.5	219.5
12/10	-	153.0	191.3	229.5	267.8	306.0	329.2	329.2	329.2	329.2

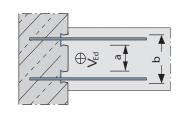
Note: Standard dimension according to page 7 and 9. Load capacities for further rebar dimensions and for rebar spacings of 25 cm, see type test.

Basis of Calculation According to Type Test and Approval no. Z-21.8-2035

# Shear load capacity, longitudinal to the case — single row profile

Multipart connection example;  $2 \times \text{Type 1}$ Standard type according to page 7





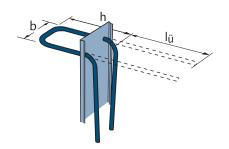
Shear load resist	Shear load resistance $V_{Rdi}$ [kN/m] ( $\leq V_{Rdi,max}$ ) Always refer to the information in the type test											
profile	b=260 mm				<b>2×HBT 55</b> (Ø8) <b>2×HBT 85</b> (Ø10,12) b=360 mm b=360 mm			Ø 10, 12) b=360 mm	b=400 mm			
rebarØ[mm]/					c	oncrete st	rength clas	s				
spacing s [cm]	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37
8/20	193.4	224.4	253.4	207.4	240.7	271.8	228.5	265.1	299.4	242.5	281.4	317.7
8/15	227.4	263.9	298.0	241.5	280.2	316.4	262.5	304.6	344.0	276.6	320.9	362.4
8/10	295.6	343.0	387.3	309.6	359.3	405.7	330.7	383.7	433.3	344.7	400.0	451.7
10/20	213.7	248.0	280.0	227.7	264.2	298.4	248.8	288.7	326.0	262.8	304.9	344.3
10/15	254.5	295.3	333.5	268.5	311.6	351.9	289.6	336.0	379.5	303.6	352.3	397.8
10/10	336.2	390.1	440.5	350.2	406.4	458.9	371.3	430.8	486.5	385.3	447.1	504.9
12/20	231.9	269.1	303.9	245.9	285.4	322.2	267.0	309.8	349.8	281.0	326.1	368.2
12/15	278.8	323.5	365.3	292.8	339.8	383.7	313.9	364.2	411.3	327.9	380.5	429.6
12/10	372.6	432.3	488.2	386.6	448.6	506.6	407.7	473.0	534.2	421.7	489.3	552.6

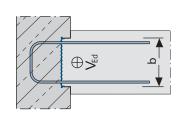
Note: Load capacities for further joint widths, for further rebar dimensions and for rebar spacings of 25 cm, see type test.

The joint areas a between the HBT Cases must be designed as rough or suitably indented as defined in DIN EN 1992-1-1; 6.2.5.

# Shear load capacity, longitudinal to the case — double-row profile

Single connection example;  $1 \times \textbf{Type 5}$  Standard type according to page 9



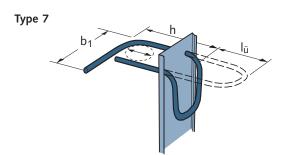


Shear load resist	Shear load resistance V <sub>Rdi</sub> [kN/m] (≤ V <sub>Rdi,max</sub> ) for HBT 120, 150, 190 and 220								Always refer to the information in the type test					
profile	HBT 120 HBT 150				HBT 190			HBT 220						
rebarØ[mm]/							rength clas							
spacing s [cm]	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37	C20/25	C25/30	C30/37		
8/20	145.0	168.2	190.0	154.8	179.6	202.9	167.4	194.3	219.4	180.1	208.9	235.9		
8/15	179.1	207.8	234.6	188.9	219.2	247.5	201.5	233.8	264.0	214.1	248.5	280.6		
8/10	247.2	286.8	323.9	257.0	298.2	336.8	269.6	312.9	353.3	282.3	327.5	369.9		
10/20	165.3	191.8	216.6	175.1	203.2	229.5	187.7	217.8	246.0	200.4	232.5	262.5		
10/15	206.1	239.2	270.1	215.9	250.6	283.0	228.6	265.2	299.5	241.2	279.9	316.0		
10/10	279.0	323.8	365.6	297.6	345.3	390.0	310.2	360.0	406.5	322.9	374.6	423.1		
12/20	183.5	212.9	240.4	193.3	224.3	253.3	205.9	239.0	269.8	218.6	253.6	286.4		
12/15	230.4	267.3	301.9	240.2	278.7	314.8	252.8	293.4	331.3	265.5	308.0	347.8		
12/10	306.3	355.5	401.4	334.0	387.6	437.7	346.6	402.2	454.2	359.3	416.9	470.8		

Note: Load capacities for the HBT 80 profile; for further rebar dimensions and for rebar spacings of 25 cm, see type test

#### Calculating Reinforced Concrete Corbel

#### Calculating a reinforced concrete corbel for HBT Type 7



## Strutural boundary conditions

Geometric assumptions:  $0.2 \le \frac{a_c}{h_c} \le 1.0$ 

Anchorage length in the corbel:

$$I_{bd,dir} = \frac{2}{3} I_{b,eq} \ge \max \begin{cases} 0.67 \cdot \alpha_1 \cdot \alpha_4 \cdot I_{b,rqd} \\ 6.7 \emptyset \end{cases}$$

# Shear load capacity in the corbel:

$$F_{Ed} \le V_{Rd,max} = 0.5 \cdot v \cdot b_c \cdot z \cdot \frac{f_{ck}}{\gamma_c}$$
where  $v = 0.7 - \frac{f_{ck}}{200 \text{ N/mm}^2} \ge 0.5$ 
 $z = 0.9 \cdot d$ 

# Tensile load in the corbel:

$$\begin{split} &Z_{Ed} = F_{Ed} \cdot \frac{a_C}{z_o} + H_{Ed} \cdot \frac{a_H + z_o}{z_o} \\ &\text{where } z_o = d \cdot \left(1 - 0.4 \cdot \frac{F_{Ed}}{V_{Rd,max}}\right); \; H_{Ed} \geq 0.2 \cdot F_{Ed} \\ &\text{and } \frac{a_C}{z_o} \; \geq 0.4 \end{split}$$

#### Required reinforcement for tension

$$a_{s,rqd} = min \begin{cases} \frac{Z_{Ed}}{0.8 \cdot f_{yd}} \\ \frac{F_{bd}}{0.8 \cdot f_{yd}} \end{cases}$$
; where  $F_{bd}$  = anchored load

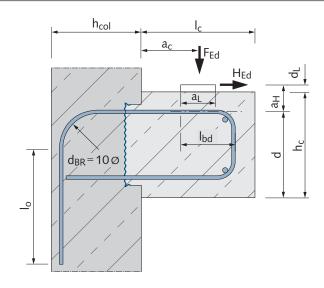
Verifying the reinforcement overlap length in the wall for:

$$I_{0,avail.} = b_1 - 6\emptyset$$

In addition to the corbel calculation above, the rebend connection has to be verified according to the specifications in approval no. Z-21.8-2035 (see page 10 f).

The compressive stresses under the load application plate and the wall's joint load bearing capacity must be verified in accordance with DIN EN 1992-1-1 resp. publication no. 532 issued by the German Committee for Structural Concrete (DAfStb).

The spacing between the side-edge of the corbel and the outermost stirrups in the HBT Connection should not exceed 5 cm. The free side-edge of the corbel must be strengthened using stirrups.



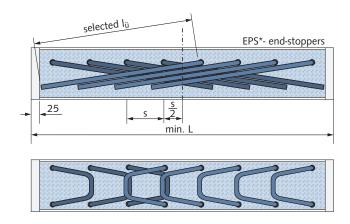
Concrete strength class C30/37 c <sub>nom</sub> =35 mm h <sub>c</sub> =										
		160 mm	190 mm	230 mm	250 mm					
	reinforce-	prce-								
	ment ⊘/s	HBT 120	HBT 150	HBT 190	HBT 220					
L <sub>c</sub> = 200 mm	8/25	51.7	61.7	61.4	61.3					
l <sub>ü</sub> = 165 mm	8/20	64.2	76.9	77.1	77.0					
a <sub>c</sub> = 100 mm	8/15	64.5	-	-	-					
	10/25	63.7	76.4	91.8	96.7					
	10/20	64.0	80.1	101.5	110.1					
	10/15	-	-	101.5	110.1					
$L_c = 220 \text{ mm}$	8/25	53.4	61.5	61.2	61.1					
I <sub>ü</sub> = 185 mm	8/20	64.4	77.3	77.0	76.8					
a <sub>c</sub> = 110 mm	10/25	63.9	79.1	95.4	96.5					
	10/20	-	79.9	101.3	110.0					
	12/25	63.3	79.4	100.8	109.6					
	12/20	-	79.4	100.8	109.6					
$L_c = 240 \text{ mm}$	8/25	53.3	61.4	61.1	60.9					
$I_{\ddot{u}} = 205 \text{ mm}$	8/20	64.3	77.1	76.8	76.7					
$a_c = 120 \text{ mm}$	10/25	63.8	79.8	96.5	96.3					
	10/20	-	-	101.2	109.8					
	12/25	63.2	79.3	100.6	109.5					
$L_c = 260 \text{ mm}$	8/25	50.1	60.5	60.9	60.7					
$I_{\ddot{u}} = 225 \text{ mm}$	8/20	62.2	-	-	-					
$a_c = 130 \text{ mm}$	10/25	63.7	79.7	96.3	96.2					
	12/25	-	79.1	100.5	109.3					
$L_c = 280 \text{ mm}$	8/25	47.3	57.2	60.8	60.6					
$I_{\ddot{u}} = 245 \text{ mm}$	10/25	-	-	96.2	96.0					
$a_c = 140 \text{ mm}$	12/25	-	79.0	-	-					

These values are for pre-dimensioning; final values must be verified. Further dimensions and resistance values are available on request. Generally, significantly better resistance values are possible when using the HALFEN HSC Stud connector.



#### Layout of the Reinforcement in the Case

# Minimal element lengths for the rebend connections — layout of the reinforcement bars in the case



*	Expanded	polystyrene

Minimal element lengths for l <sub>ü</sub> standard								
Ø [mm]	rebar spacing s [cm]	① l <sub>ü</sub> [mm]	minimal case length L [mm]	number of rebar				
8	10	320	650	6				
8	15	320	600	4				
8	20	320	650	4				
10	10	390	800	8				
10	15	390	750	4				
10	20	390	700	4				
12	10	460	950	8				
12	15	460	900	6				
12	20	460	850	4				
① Max lii-	① Max. lii - observe dimensions for selected profile (see page 7 and 9)							

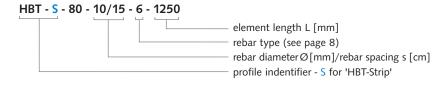
# HBT Strip — the perfect solution for precast elements

Due to inherent low concrete cover and restriction in element size in precast elements, demands on precise measurement and manufacturing of rebend connections are generally higher.

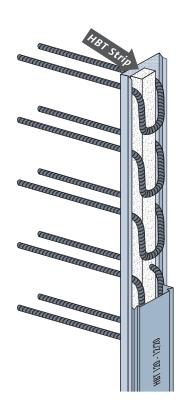
The HBT-Connection with 'HBT Strip' (HBT-S) fulfils these requirements perfectly. Available for all double-row HBT connections with rebar types 6, 7, 8 and 9, in profile widths HBT - \$ 80, 120, 150, 190, 220.

- · rebars are secured during transport
- compliance with required over-lap and anchorage lengths
- recommended for loops and corbel connections
- HBT Strip is easily removed after striking the formwork

#### Order example:



Installation is as described on page 18; the HBT Strip must be removed after all connecting rebar have been rebent.



**Application Suggestions** 

### Application of short elements

#### Various lengths - efficient installation times

Combining 1250 mm standard elements and 800 mm elements helps to avoid unnecessary modification of HBT Elements.

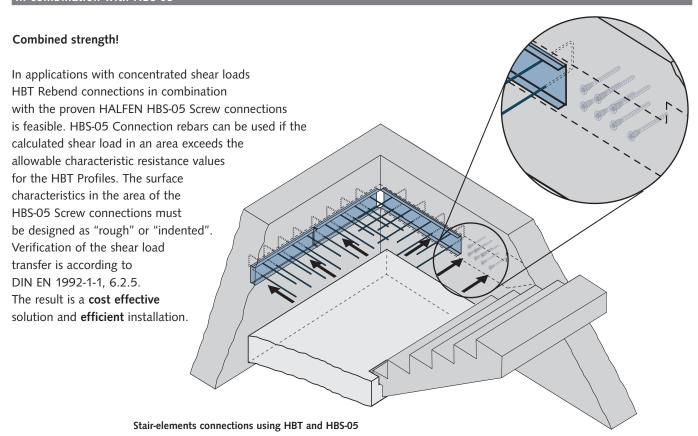
Standard combinations									
	clear floor height [m]								
HBT Element	2.40	2.50	2.85	3.30	3.75				
	combinations								
short element L = 800 mm	3	-	2	1	-				
standard element L = 1250 mm	-	2	1	2	3				

In numerous applications, further on-site modification of the HBT cases is therefore not required. The risk of damaging the rebar in the casing is avoided. The planner can plan more efficiently and on-site preparation time for installation of the HBT Elements is reduced.

1250 mm

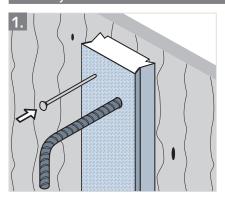
Wall connection using two 1250 mm HBT Elements and a 800 mm HBT Element to obtain 3.30 m floor height

#### In combination with HBS-05

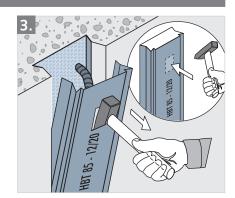


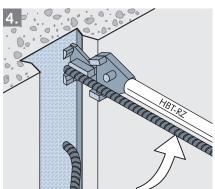
#### Assembly Instructions

#### Assembly instructions









- 1. Nail the HBT Element to the timber formwork in the specified position. Use suitable methods for fixing elements to metal formwork, for example magnets. Check stirrup lengths protruding from the case after installation.
- 2. After the concrete has cured, strike the formwork; hitting a wood block placed in the long groove in the cover with a hammer, loosen the cover.
- 3. Use a hammer with a claw to punch-in the perforated hole in the cover; hook the hammer in the hole and pull the lid out.
- 4. Place the HALFEN Rebending tool under the rebar and pull down on the handle evenly with both hands until the rebar is in the correct position. The bar must be rebent straight without any kinks. Proceed in a similar manner to rebend all bars in the HBT Element one-by-one. The profiled back of the HBT Element case remains in the concrete.

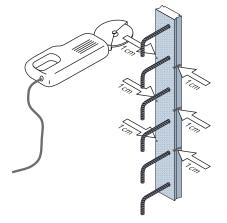
Note: A more detailed installation instruction can be found at www.halfen.com.

# HBT Element adapted to curved formwork

# Adapting the HBT Case

Using an angle-grinder cut approximately 1 cm deep incisions symmetrically into both sides of the case at regular intervals; the HBT housing loses its rigidity, easing fixing to the formwork. To achieve a better fit to smaller curvature (< 3.00 m), up to seven incisions per side are possible.

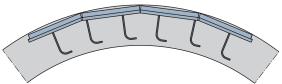
After fixing the HBT Case to the formwork cover the incisions with adhesive tape to prevent concrete seeping into the form.





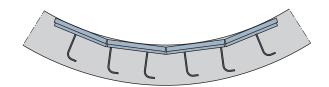
Caution when working with an angle-grinder!

The reinforcing steel bars in the HBT Case must not be damaged.



HBT Element fitted to an convex curvature

Outer radius ≥ ca. 3.00 m; smaller radius is achieved with more incisions.



HBT Element fitted to an concave curvature Inner radius ≥ ca. 3.00 m; smaller radius is achieved with more incisions.

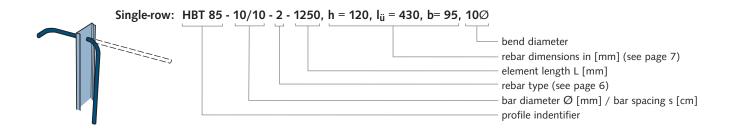
#### Order Examples

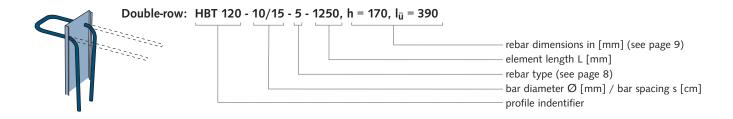
# Order examples

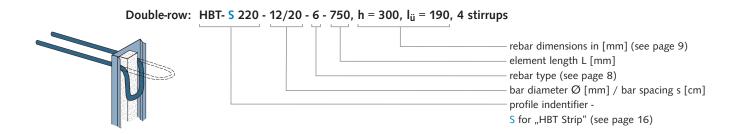
HALFEN offers a wide range of standard HBT Rebend connections for the most common applications (see page 6ff.).

There is also a wide product range with corresponding profile widths and rebar shapes to choose from.

Rebar dimensions and element lengths are freely definable, limited only by geometric specifications and limits in production (see also page 16).



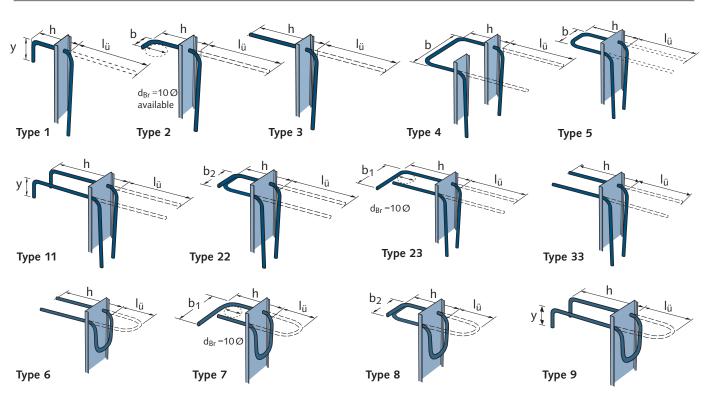




HBT Connection with foam strip filler "HBT Strip". (see page 16)

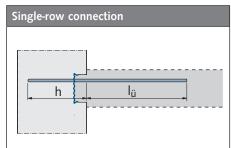
#### Order form

# Single and double-row connections

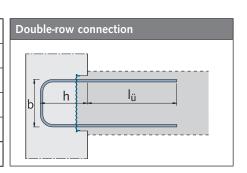


Pos.	profile HBT	rebar Ø [mm]	rebar spacing <b>s</b> [cm]	stirrup type	element length <b>L</b> [mm]	<b>h</b> [mm]	l <sub>ü</sub> [mm]	1×1	value if pre	in[mesent	nm]	d <sub>Br</sub>	total length [m]	number of elements [items]

Note: do not exceed  $l_{\ddot{u},max}$  see table on page 7 and 9; see page 6 to 9 for rebar and profile dimensions.



Company	Fax or email
Address	this form to
City, Postcode/Z	ip. HALFEN.
Tel. no.	See back-cover
E-Mail	for addresses.
Fax	



Tender Texts

#### HALFEN Rebend connection, type HBT 85-10/20-1-1250

HALFEN HBT Rebend connection with single-row rebend reinforcement in a galvanised sheet-metal case to form a reinforced connection, with general building authority approval no. Z-21.8-2035,

```
Type HBT 85 – 10/20 – 1 – 1250
with

85 = type identifier for a case width of 85 mm with a single-row of rebar,

10/20 = reinforcement steel B500B with 10 mm bar diameter and 200 mm bar spacing,

1 = standard rebar type 1,

1250 = element length [mm],

in standard rebar dimensions

rebar length 1 h = 170 mm,

rebar length 2 | | | | | | | | | | | | | | | |

bend length y = 95 mm,
```

or equivalent; deliver and install according to the manufacturer's instructions.

# HALFEN Rebend connection, type HBT 150-12/15-5-1250

HALFEN HBT Rebend connection with double-row rebend reinforcement in a galvanised sheet-metal case to form a reinforced connection, with general building authority approval no. Z-21.8-2035,

```
Type HBT 150 – 12/15 – 5 – 1250
with

150 = type identifier for a case width of 150 mm with a double-row of rebar,

12/15 = reinforcement steel B500B with 12 mm bar diameter and 150 mm bar spacing,

5 = standard rebar type 5,

1250 = element length [mm],

in standard rebar dimensions

rebar length 1 h = 170 mm,

rebar length 2 l<sub>ü</sub> = 460 mm,
```

or equivalent; deliver and install according to the manufacturer's instructions.

# HALFEN Rebend connection, with stainless steel reinforcement bars

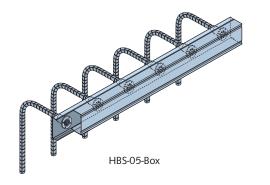
Use the text as above, but replace "B500B" with "stainless steel B500 NR according to building authority approval".

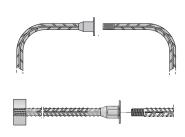
Further tender texts can be found at www.halfen.com.

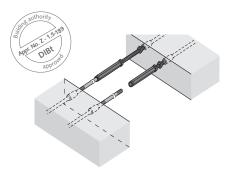
#### **Further HALFEN Reinforcement Products**

#### HALFEN HBS-05 Screw connections

The HALFEN HBS-05 Screw connections allow rebar continuity joints possible with simple screw and socket rebar. Their versatility allows nearly every type of reinforcement joint. HALFEN HBS-05 fulfils German and international certification criterion. Extensive certification and test reports prove their suitability also under extreme conditions.





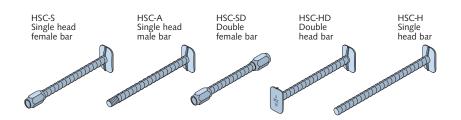


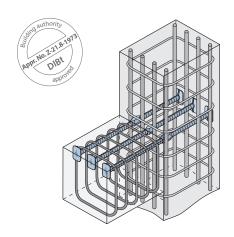
No torque wrench or special tools required – visual check is sufficient

#### HALFEN HSC Stud Connector

The HALFEN HSC Stud connector is a building authority approved reinforcement optimized for anchorage in concrete. Maximum exploitation of the reinforcement is possible with extremely short anchoring lengths.

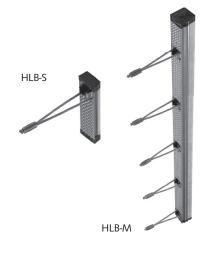
HALFEN HSC Anchor is especially advantageous for use in high-density reinforced areas, for example; corbels and frame corner nodes.

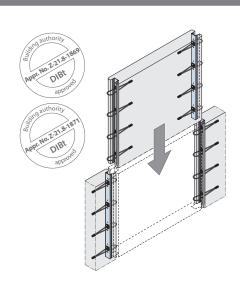




#### HALFEN HLB Loop Box

The HALFEN HLB Loop Box is an efficient and time saving method for connecting concrete elements. HLB Loop boxes are cast into concrete elements (e.g. wall elements) in the precast plant. The prepared concrete elements are transported to site, lifted and correctly placed with a crane; the joint between the elements is then cement-grouted.





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#### References



Kö-Bogen, Düsseldorf, Germany

The Kö-Bogen complex designed by Daniel Libeskind offers commercial space for business as well as a shopping centre with restaurants etc. The Kö-Bogen (curve) is located at the end of the Königsallee in Düsseldorf, Germany.

HALFEN supplied HALFEN HBT Rebend connections for the Kö-Bogen. Other HALFEN products also used in this project include;

- HALFEN HTA Channels
- HALFEN Masonry connection channels
- HALFEN HDB Shear rails
- HALFEN HBS-05 Screw connections



Kopernikus Science centre, Warsaw, Poland

The Science centre in Warsaw, Poland, named after Nikolaus Kopernikus, is a collection of buildings with six exhibition areas.

HALFEN supplied HALFEN HBT Rebend connections for this project.



DATEV IT-Campus, Nuremberg, Germany

The DATEV IT-Campus on Fürther Strasse in Nuremberg offers 1800 workspaces including 200 individual offices and conference rooms of various sizes.

The building was inaugurated in 2015.

HALFEN supplied HDB Shear rails and HBT Rebend connections for this project.

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